

**A LABORATORY STUDY ON THE CONSOLIDATION
CHARACTERISTICS OF MARINE CLAY TREATED WITH CALCIUM
CARBIDE POWDER AND ADDITIVES**

Dr. D. Koteswara Rao¹, Y.Saieswar²

¹Professor of Civil Engineering & Director, Faculty Development Centre, JNTUK Kakinada, A.P, India

²P.G Student, Department of Civil Engineering, University College of Engineering,
JNTUK Kakinada, A.P, India

Abstract— Rapid Population growth results in scarcity of land for housing, commercial, industrial, transportation and infrastructure purpose. This necessitated the use of land, which has weak strata, where the geotechnical engineers are challenged by the presence of problematic soils with varied engineering properties. One of such soil is marine clay. Marine Clays are problematic soils all around the world and especially in the regions where climate is arid or semi-arid. These soils are highly compressible soils suffering from low shear strength and high compressibility associated with movements of pavements and foundations may result in cracking and breaking of structures. Several methods are used to improve the engineering properties of marine clay. Among those methods, chemical stabilization is widely used. In this present study the consolidation characteristics i.e., coefficient of consolidation, compression index and rate of consolidation of untreated and treated marine clay were determined by adding Calcium Carbide Powder as admixture and also with chemicals i.e. ammonium chloride, potassium chloride and ferric chloride.

Key words— Marine clay, chemical stabilisation, Calcium Carbide Powder, NH_4Cl , KCl , $FeCl_3$, Consolidation Characteristics.

I. INTRODUCTION

The soil found in the ocean bed is classified as marine clay. It can even be located onshore as well. The marine clay is available at fully saturated condition in the costal corridors. Marine clay is microcrystalline in nature and clay minerals like chlorite, Kaolinite and Illite and non-clay minerals like quartz and feldspar are moderately present in the soil. The soils have higher proportion of organic matters that acts as a cementing agent. The natural moisture content of the marine clays is always greater than its liquid limit. The particles can self-assemble in to various configurations, each with totally different properties. Marine clay has low shear strength and high compressibility. Hence, it is becoming a great challenge for the Civil Engineers to design suitable pavements and foundation for the structures in these regions. The problems posed by marine clay can be minimised by various ground improvement techniques.

Methods to Improve the Marine clay : Different measures have been proposed and methodologies adopted for overcoming problems associated with these kinds of soils. The methodologies used worldwide are like pre-wetting, surcharge loading, admixture and chemical stabilization, reinforcement techniques, sand drains etc., Among these, one of the most widely used methods are using admixture, chemical and reinforcement techniques for improving the load carrying capacity of the soft soils. In this experimental investigative, marine clay is stabilised with the percentage variation of calcium carbide powder as an admixture along with chemicals viz ammonium chloride, potassium chloride and ferric chloride which are added separately at optimum percentages and the consolidation characteristics are noted.

II. REVIEW OF LITERATURE

The comprehensive review of literature, carried out worldwide since last five decades shows that a considerable amount of work is related for the determination of consolidation characteristics, deformation characteristics and strength characteristics of the marine clay. From the various investigations carried out on consolidation characteristics and strength characteristics of marine clay conducted by Narasimha Rao et.al (1987,1996), Shridharan et.al (1989), Mathew et.al (1997) and G.Rajasekaran et.al (2002) are worthy to be noted. Improving the strength of marine clay by the stabilisation technique was performed by Nontanandh et.al (2004) and an accountable study on engineering properties of marine clay was performed by Dr. D. Koteswar Rao (2006,2011,2012,2013, 2014).

III. OBJECTIVES OF STUDY

- To determine the properties of marine clay.

- To evaluate performance of marine clay treated with Calcium Carbide Powder, NH₄Cl, KCl and FeCl₃.
- To evaluate the consolidation characteristics of marine clay treated with Calcium Carbide Powder, NH₄Cl, KCl, and FeCl₃.

IV. MATERIALS USED

A. Marine Clay (MC)

The soil used for the study was collected at a depth of 0.5 to 1m below the bed level in Kakinada Sea Port Limited, Kakinada. The Index & Engineering properties of marine clay are determined as per IS code of practice.

B. Calcium Carbide Powder (CaC₂)

Calcium Carbide Powder (CCP) is a by-product from the acetylene gas production. This gas is used around the world for welding, lighting, metal cutting and to ripen fruits.



Calcium hydroxide rich material used to produce moderately high strength. Three basic reactions, which are cation exchange, aggregation and flocculation which attributed the engineering properties of rich material stabilized clay. The influences of these three are primarily responsible for the change in plasticity and shrinkage. The quantity of Calcium Carbide Powder (CaC₂) was varied from 4 to 12% to the dry weight of soil.

C. Ammonium Chloride (NH₄Cl)

Commercial grade Ammonium Chloride (NH₄Cl) was used in this study. Ammonium Chloride (NH₄Cl) in varying percentages of 0.4%, 0.6%, 0.8%, 1.0% & 1.2% is added to the marine clay. The quantity of Ammonium Chloride (NH₄Cl) was varied from 0 to 1.2% to the dry weight of soil.

D. Potassium Chloride (KCl)

Commercial grade Potassium Chloride (KCl) was used in this study. Potassium Chloride (KCl) in varying percentages of 0.6%, 0.8%, 1.0%, 1.2% & 1.4% is added to the marine clay. The quantity of Potassium Chloride (KCl) was varied from 0.6% to 1.4% to the dry weight of soil.

E. Ferric Chloride (FeCl₃)

Commercial grade Ferric Chloride (FeCl₃) was used in this study. Ferric Chloride (FeCl₃) in varying percentages of 0.6%, 0.8%, 1.0%, 1.2% & 1.4% is added to the marine clay. The quantity of Ferric Chloride (FeCl₃) was varied from 0.6% to 1.4% to the dry weight of soil.

V. LABORATORY INVESTIGATION

The Physical Properties of marine clay and particle size distribution of marine clay were presented in tables-1 below. The tests are conducted as per IS 2720 codes of practice.

TABLE I
Physical properties of Untreated Marine Clay

S.no	Property	Values	
1.	Gravel (%)	0.0	
2.	Sand (%)	6.0	
3.	Fines	Silt (%)	24.65
		Clay (%)	69.35
4.	Liquid Limit (%)	W _L	76
5.	Plastic Limit (%)	W _P	34.33
6.	Plasticity Index (%)	I _P	41.67
7.	Soil Classification	--	CH
8.	Specific Gravity	G	2.4
9.	Free Swell (%)	FS	80
10.	Optimum Moisture Content (%)	OMC	37.32
11.	Maximum Dry Density (g/cc)	MDD	1.47
12.	CBR (%)	--	1.792
13.	Natural Moisture Content (%)	--	82

Compaction Test:

The tests were conducted as per IS 2720 Part – VII. OMC & MDD of air dried untreated and treated Marine clay with calcium carbide powder, NH₄Cl, KCl & FeCl₃ are presented in the tables II, III, IV, V, VI & Figures below.

TABLE II
 OMC & MDD Values of Untreated Marine Clay

S.no	Water Content (%)	Dry Density(g/cc)
1.	33.12	1.423
2.	34.61	1.448
3.	37.32	1.470
4.	38.67	1.435
5.	39.41	1.419

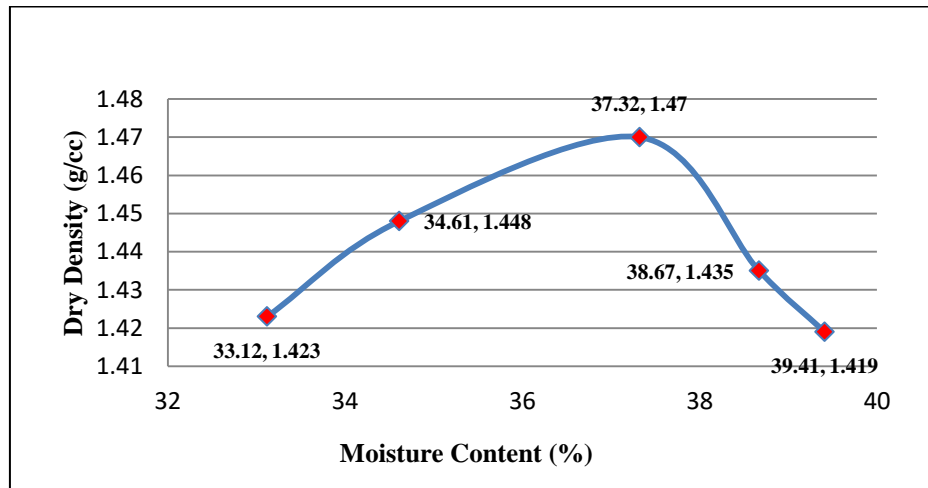


Fig.1 OMC & MDD Values of Untreated Marine Clay.

TABLE III
 OMC & MDD Values of Marine Clay treated with % Variation of Calcium Carbide Powder

Marine clay treated with % variation of CCP	Water Content (%)	Dry Density(g/cc)
4	27.12	1.42
8	27.85	1.48
12	28.13	1.53
16	28.92	1.49
20	29.52	1.43

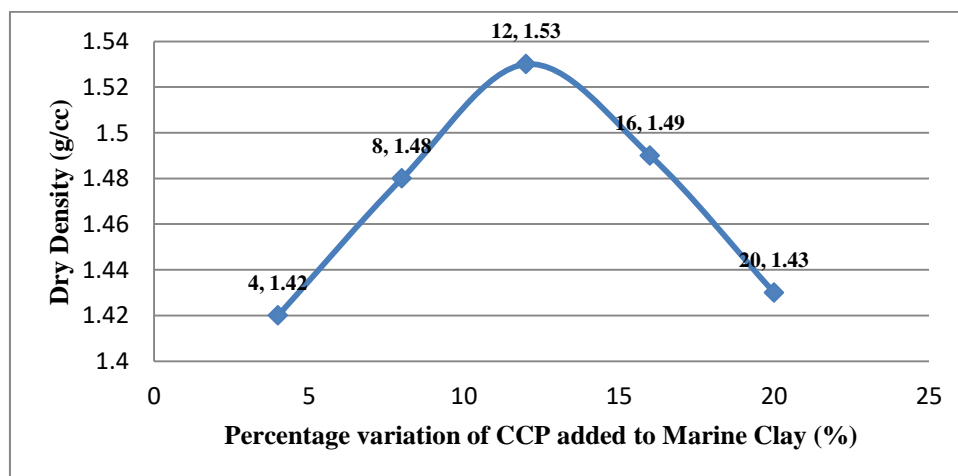


Fig. 2 OMC & MDD Values of Marine Clay treated with Calcium Carbide Powder

It is observed from the above Table III and Fig.2, the treated Marine clay has exhibited the MDD value of 1.53 g/cc on addition 12% CCP.

TABLE IV

OMC & MDD Values of Marine Clay treated with 12% CCP along with percentage variation of NH_4Cl

CCP treated marine clay with % variation of NH_4Cl	Water Content (%)	Dry Density(g/cc)
0.4	24.14	1.54
0.6	24.84	1.59
0.8	25.23	1.65
1.0	25.92	1.59
1.2	26.30	1.52

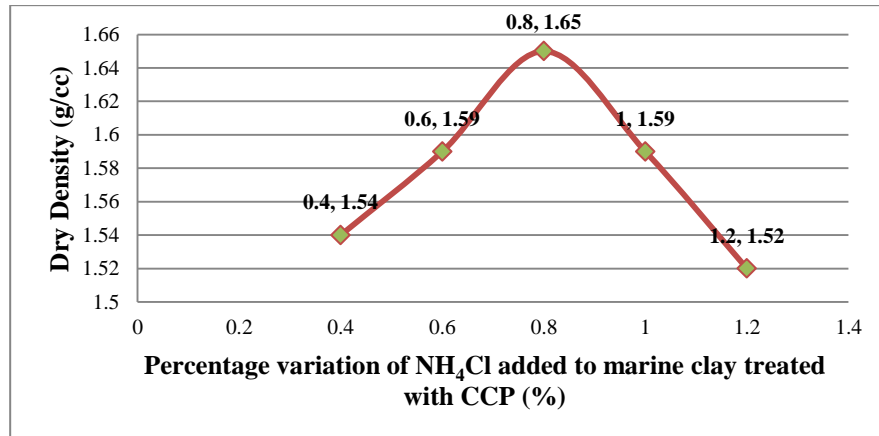


Fig. 3 OMC & MDD Values of Marine Clay treated with 12% CCP along with percentage variation of NH_4Cl

It is observed from the above Table IV and Fig.3, the treated Marine clay has exhibited the MDD value of 1.650 g/cc on addition 12% CCP and 0.80% of NH_4Cl

TABLE V

OMC & MDD Values of Marine Clay with optimum percentage of CCP and mixed with percentage variation of KCl

CCP treated marine clay with % variation of KCl	Water Content (%)	Dry Density(g/cc)
0.6	23.23	1.58
0.8	23.92	1.63
1.0	24.38	1.68
1.2	24.86	1.62
1.4	25.37	1.58

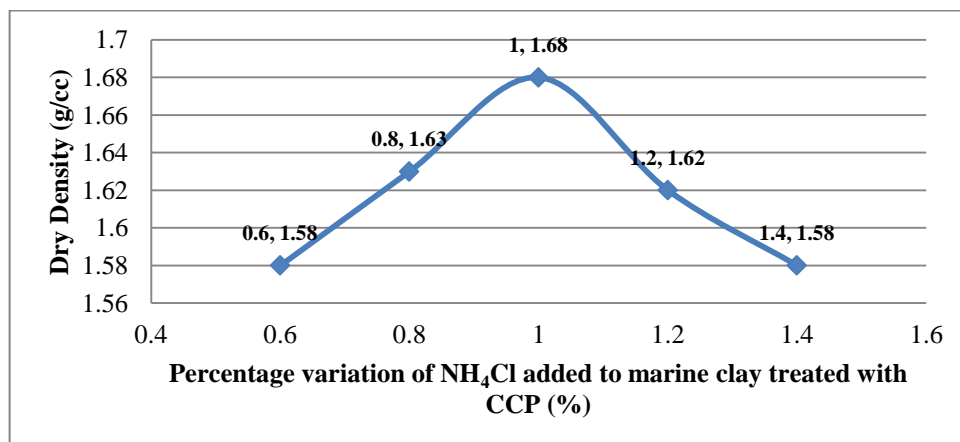


Fig. 4 OMC & MDD Values of Marine Clay with 12% of CCP and mixed with percentage variation of KCl

It is observed from the above Table V and Fig.4, the treated Marine clay has exhibited the MDD value of 1.68 g/cc on addition of 12% CCP and 1% KCl.

TABLE VI

OMC & MDD Values of Marine Clay with optimum percentage of CCP and mixed with percentage variation of FeCl₃

CCP treated marine clay with % variation of FeCl ₃	Water Content (%)	Dry Density(g/cc)
0.6	21.15	1.69
0.8	21.92	1.74
1.0	22.55	1.79
1.2	23.06	1.72
1.4	23.86	1.68

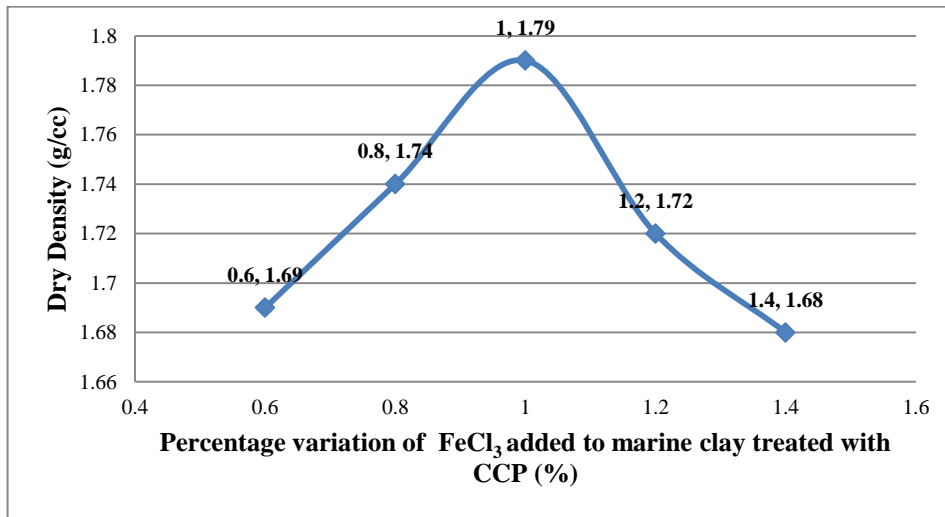


Fig.5: OMC & MDD Values of Marine Clay with optimum percentage of CCP and mixed with percentage variation of FeCl₃

It is observed from the above Table VI and Fig.5, the treated Marine clay has exhibited the MDD value of 1.79 g/cc on addition of 12% CCP and 1% FeCl₃.

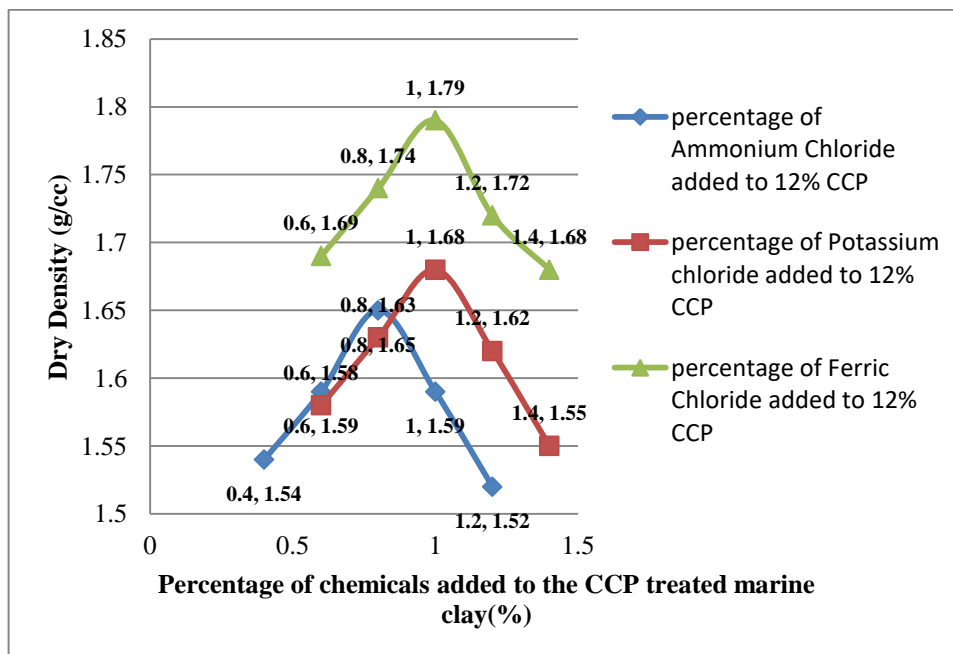


Fig. 6 Chart showing all the optimum values of chemicals added to the optimum Calcium Carbide Powder

CONSOLIDATION TEST:

The test was carried out in accordance with IS – 2720 part 15 codes of practice. The calculations of consolidation was carried out by using square root of time method. Which is one of the curve fitting methods as per the codes of practice.

Consolidation characteristics:

The time required for the 90% consolidation, coefficient of consolidation & compression index of untreated and treated marine clay by using a standard method and were presented in table VII & Fig. 7 to 11.

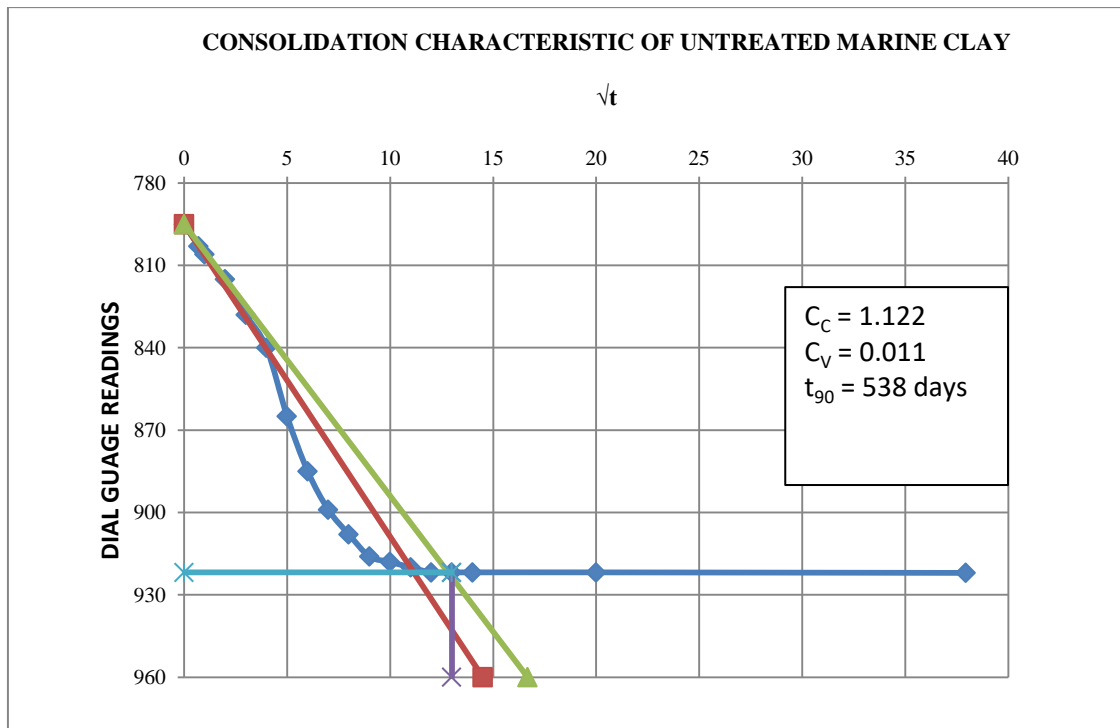


Fig.7 Consolidation Characteristic Of Untreated Marine Clay @3.2Kg/Cm

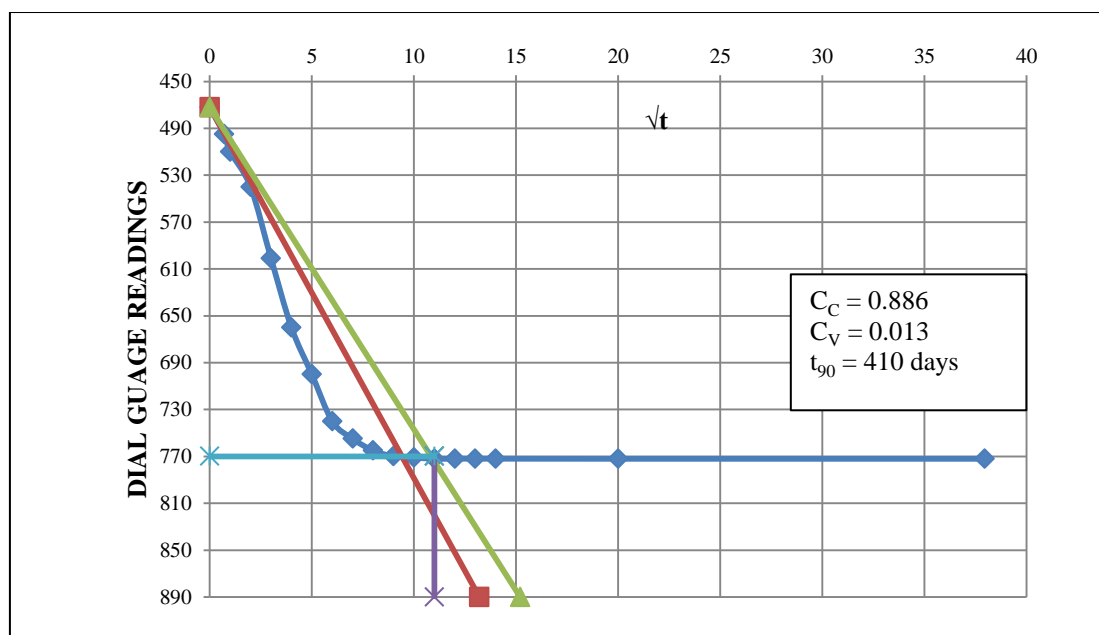


Fig. 8 Consolidation Characteristics of Marine Clay treated with 12% of Calcium Carbide Powder

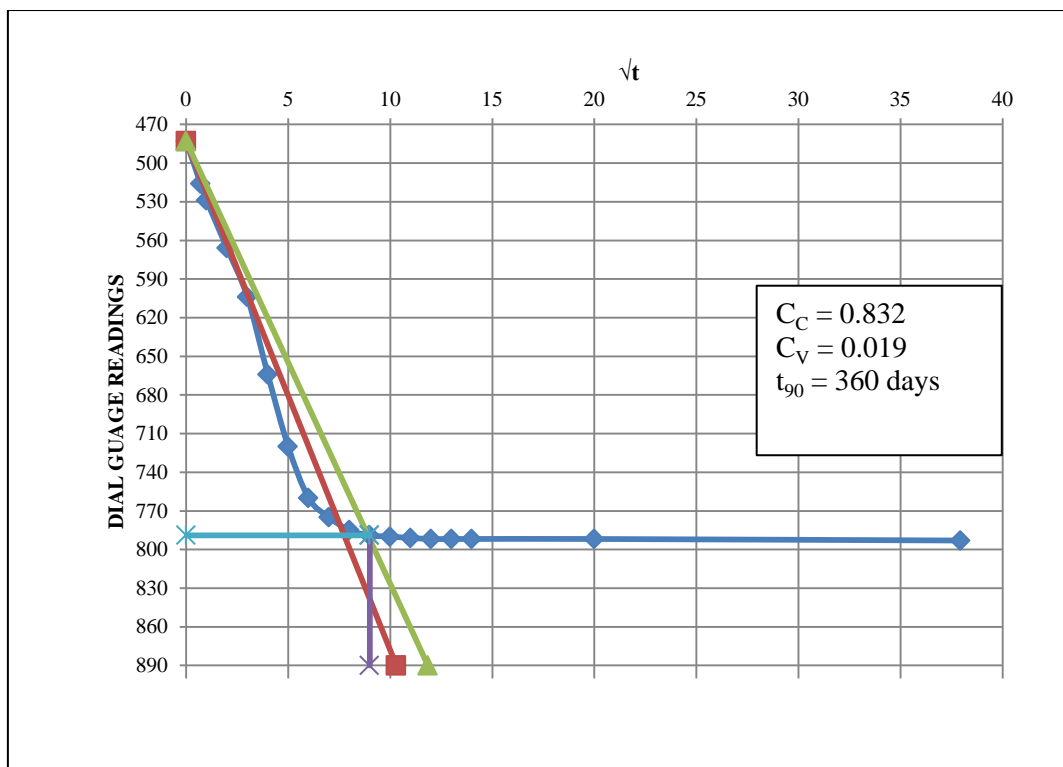


Fig. 9 Consolidation Characteristics of Marine Clay treated with optimum percentage i.e., 12% CaC_2 and 0.8% NH_4Cl

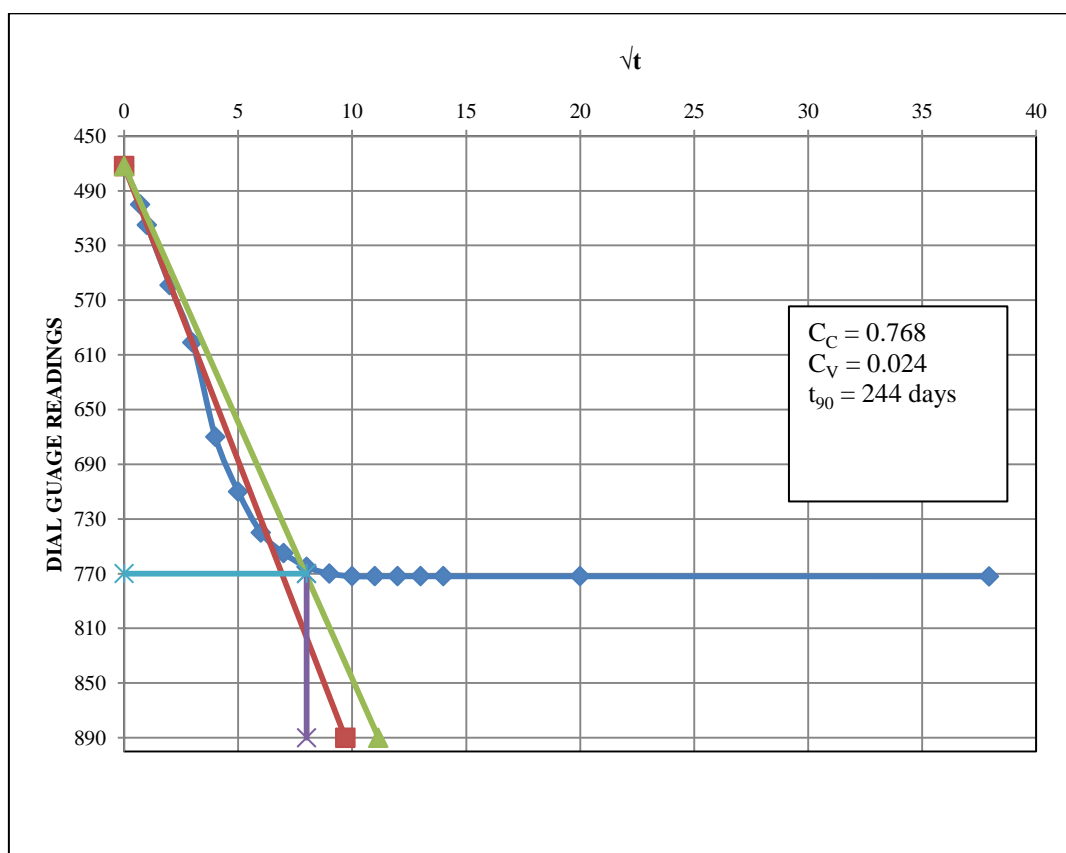


Fig. 10 Consolidation Characteristics of Marine Clay treated with optimum percentage i.e., 12% CaC_2 and 1% KCl

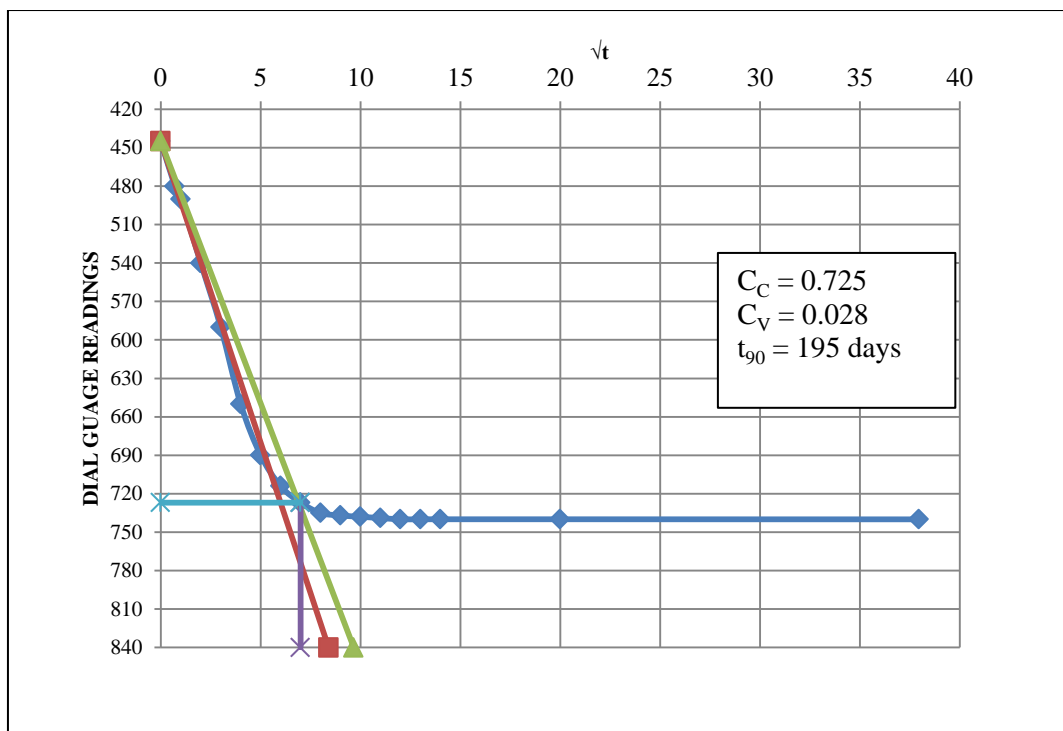


Fig. 11 Consolidation Characteristics of Marine Clay treated with optimum percentage i.e., 12% CaC_2 and 1% FeCl_3

TABLE VII

Coefficient Of Consolidation, Compression Index, Rate Of Consolidation Of Untreated And Treated Marine Clay

Description	Coefficient of consolidation C_v (cm / sec)	Compression index C_c	Rate of consolidation (days)
MC	0.011	1.122	538
MC treated with 12% CaC_2	0.013	0.886	410
MC treated with 12% CaC_2 and 0.8% NH_4Cl	0.019	0.832	360
MC treated with 12% CaC_2 and 1% KCl	0.024	0.768	244
MC treated with 12% CaC_2 and 1% FeCl_3	0.028	0.725	195

TABLE VIII

Represent the properties of Untreated Marine Clay, Marine Clay treated with 12% CCP, 0.8% NH₄Cl, 1% KCl and 1% FeCl₃

S.No	Property	MC	MC treated with 12% CCP	MC treated with 12% CCP & 0.8% NH ₄ Cl	MC treated with 12% CCP & 1% KCl	MC treated with 12% CCP & 1% FeCl ₃
1	Liquid limit (%) W _L	76	58.3	54.3	45.6	38
2	Plastic Limit (%) W _p	34.3	22.9	22.3	21.6	23
3	Plasticity Index (%) I _p	41.6	35.4	32	24	15
4	Soil classification	CH	CH	CH	CI	CI
5	Specific Gravity G	2.4	2.62	2.64	2.68	2.73
6	Optimum Moisture Content (%) O.M.C	37.3	28.13	25.23	24.38	22.55
7	Maximum Dry Density (g/cc) M.D.D	1.47	1.53	1.65	1.68	1.79
8	CBR Value (%)	1.7	6.82	7.8	8.4	8.6
9	Free Swell DFSI	80	45	35	30	25

VI. CONCLUSION

- The optimum percentage of chemicals and admixtures, observed during the laboratory investigation are summarized and presented in the following table.

S. no	Additive	Optimum percentage
1	CaC ₂	12%
2	NH ₄ Cl	0.8%
3	KCl	1%
4	FeCl ₃	1%

- Conclusions of the various results from the laboratory studies were presented.
 - It was observed from the laboratory investigations that the liquid limit and the plasticity index of marine clay has been improved by 23.26%, 15.04%; 28.55%, 23.2%; 40%, 42.4%; 50%, 64% with the addition of 12% CaC₂; 12% CaC₂ + 0.8% NH₄Cl; 12% CaC₂ + 1% KCl; 12% CaC₂ + 1% FeCl₃ respectively when compared with the untreated marine clay.
 - Future it was noticed from the laboratory investigations that the coefficient of consolidation of marine clay has been improved by 39.82%; 106.58%; 163.47%; 245.2% with the addition of 12% CaC₂; 12% CaC₂ + 0.8% NH₄Cl; 12% CaC₂ + 1% KCl; 12% CaC₂ + 1% FeCl₃ respectively when compared with the untreated marine clay.

- iii) It was concluded from the laboratory investigations that the coefficient of compression has been improved by 366.4%; 424%; 383.2%; 359% with the addition of 12% CaC₂; 12% CaC₂ + 0.8% NH₄Cl; 12% CaC₂ + 1% KCl; 12% CaC₂ + 1% FeCl₃ respectively when compared with the untreated marine clay.
- iv) It was perceived from the laboratory investigation that the rate of consolidation of marine clay has been improved by 28.32%; 51.2%; 61.7%; 70.98% with the addition of 12% CaC₂; 12% CaC₂ + 0.8% NH₄Cl; 12% CaC₂ + 1% KCl; 12% CaC₂ + 1% FeCl₃ respectively when compared with the untreated marine clay.

REFERENCES

- [1] Anandarajah. A and Chu. J (1997), Laboratory Determination of shear strength parameters for marine clay, Journal of the Institution of Engineers, SINGAPORE, VOL.14, NO.3, PP 39-46.
- [2] Balasubramaniam .A.S., Uddin, K., Sanmugarasa. K., Lee. Y.H., Oh.Y-N. (2003), Effects of Additives on Soft Clay behaviour”. Proc. of the 21st ARRB and 11th Road Engineering Association of Asia and Australia (REAAA) Conference, Cairns, Queensland, Australia, Paper No.56 (CD-ROM).
- [3] Balasubramaniam, A.S., Bergado, D.T., Buensuceso, B.R. and Yang, W.C (1989), Strength and deformation characteristics of lime treated soft clays, Geotechnical Engineering (AIT), 20, 1989, pp. 49-65.
- [4] Basack and Purkayastha (2009), Engineering properties of Marine Clays from the eastern coast of India. Journal of Engineering and Technology Research Vol.1 (6), pp. 109-114, September, 2009.
- [5] BuddhimaIndraratna and appu.S.Satkunaseelan and Mohammad G. Rasul (1991), Laboratory Properties of a soft Marine Clay reinforced with Woven and Nonwoven Geotextiles, Geotechnical Testing Journal, pp 288-295, 1991.
- [6] Chang. M. F (1991), Stress History of Singapore Marine Clay, Journal of Geotechnical Engineering Division, ASCE, Vol.22, pp 5-21.
- [7] Chu,J, Myint Win Bo, M.F.Chang and V. Choa (2002), Consolidation and Permeability Properties of Singapore Marine Clay. Journal of Geotechnical and Geo environmental Engineering, Vol.128, No.9, September 2002, pp.724-732.
- [8] Dr. D. Koteswara Roa (2013), “A laboratory Investigation on the affect of vitrified Polish Waste for improving the properties of marine clay”, International Journal of Engineering and Innovative Technology (IJEIT) ,vol- 2, Issue-11, May 2013,pp 37-41.
- [9] Dr. D. Koteswara Rao et.al (2011), Field studies on the marine clay foundation soil beds treated with lime, GBFS and reinforcement technique, IJEST, Vol.3, No.4, April 2011.
- [10]Dr. D. Koteswara Rao et.al (2011), The affect of reinforcement on GBFS and lime treated marine clay for foundation soil beds, IJEST, Vol.3, No.3, March 2011.
- [11]Dr. D. Koteswara Rao et.al (2012), A laboratory study in the affect of rice husk ash & lime on the properties of marine clay, IJEIT, ISO 9001:2008,ISSN :2277-3754, Vol. 2, Issue 1, July 2012.
- [12]IS 2720-Part II,1973 Methods of test for soils: Part 2 Determination of water content
- [13]IS: 2720-Part III, Section I, 1980, Determination of Specific Gravity.
- [14]IS: 2720-Part IV, 1975, Determination of Grain Size Distribution.
- [15]IS: 2720-Part V, 1970, Determination of Liquid Limit and Plastic Limit.
- [16]IS: 2720-Part VII, 1980, Determination of Water Content, Dry Density Relation Using Light Compaction.
- [17]IS 2720-Part VII,1983, Determination of water content dry density relation using heavy compaction
- [18]IS: 2720-Part XV, Determination of Consolidation characteristics.
- [19]IS: 2720-Part XVI, Detemination of CBR values.
- [20]Matchala Suneel, Lee Keun Park and Jong Chul Im (2008), Compressibility Characteristics of Korean Marine Clay, Marine Geo-resources & Geo technology, Vol.26, Issue 26, April 2008, pp 111-127.
- [21]Narasimha Rao, S and Swamy, K. K. R (1984), Geotechnical Properties of Indian Marine Clays, Ind. Geo-tech conf., Calcutta, Dec 84, Vol.1.
- [22]Oh, E.Y. N and Chai, G.W.K (2006), Characterization of Marine clay for Road Embankment Design in Coastal Area, Proceedings of Sixteenth international offshore and Polar Engineering Conference (ISBN 1-880653-66-4), San Francisco, USA, Vol.2, 560-563.
- [23]Sabat, A.K. (2012). “Stabilization of expansive soil using waste ceramic dust” Electronic Journal of Geotechnical Engineering, Vol. 17, Bund. Z, 3915-3926.
- [24]Shridharan, A, Rao, S.M and Chandrasekaran, S (1989), Engineering Properties of Cochin and Mangalore Marine Clays, Indian Geo-tech, J.1:265-278.

BIOGRAPHIES

Author 1: Dr.D Koteswara Rao is working as a Professor of Civil Engineering & Director, Faculty Development Centre, Jawaharlal Nehru Technological University Kakinada. He is a “ Triple Hat-Trick Best Teacher Awardee” from the Department of Civil Engineering, University College of Engineering, JNTUK Kakinada. He was awarded the “University Meritorious teacher award -2013” by the University and also received National Best Teacher award-2013. Recently he has received “The State Best Teacher Award-2017 by the Government of Andhra Pradesh, India.

He has published 33 research and review papers in various International journals and conferences. He has guided about 50 post graduate projects and four research scholars are working his guidance. He is a leading consulting member in the fields of Surveying, Transportation and Geotechnical Engineering.



Dr. D. Koteswara Rao, Professor of Civil Engineering & Director, Faculty Development Centre, University College of Engineering, JNTUK KAKINADA, Andhra Pradesh, India.

Author 2:



Y.Saieswar , PG student of Soil Mechanics and Foundation Engineering, Department of Civil Engineering, UCEK, JNTUK Kakinada, East Godavari District-533003, Andhra Pradesh, India.