

International Journal of Technical Innovation in Modern Engineering & Science (IJTIMES)

> Impact Factor: 3.45 (SJIF-2015), e-ISSN: 2455-2585 Volume 4, Issue 02, February-2018

EFFECT OF SUPPLEMENTAL DAMPING ON LRB AND FPS SEISMIC ISOLATORS UNDER NEAR-FAULT GROUND MOTIONS

K. Sandeep Kumar¹, Dr. E. ArunaKanthi²

¹M. Tech (Computer aided structural Engineering), Department of Civil Engineering, JNTUA College of *Engineering , Ananthapuramu. 515002, India.*

² Associate Professor of Civil Engineering , JNTUA College of Engineering , Ananthapuramu. 515002, India.

ABSTRACT -Numerical simulations are performed to assess the effects of near-fault ground motions on baseisolated buildings that consist of either lead-rubber (LRB) or friction-pendulum system (FPS) bearings in addition to supplemented viscous dampers. While LRB and FPS isolation systems have been applied for a number of years, the addition of supplemental damping devices is being currently considered for strong ground motions to reduce the isolator displacements. However, the main problem in this case is that the addition of damping may increase both internal deformation and absolute accelerations of the superstructure and thus may defeat many of the gains for which base isolation is intended.

In the present paper, a detailed and systematic investigation on the performance of LRB and FPS isolation systems, provided with supplemental viscous damping under the effect of near-fault ground motions, will be carried out by using commercial finite element software.

In the present analysis, a residential building with 20 floors is to be analyzed with columns, columns with LRB and FPS isolation systems. The building comes under zone 2 & zone 5. Moments, Storey Shear, Drift and Torsion will be compared for all the cases.

Earthquake load is becoming a great concern in our country as because not a single zone can be designated as earthquake resistant zone. One of the most important aspects is to construct a building structure, which can resist the seismic force efficiently. Study is made on the structural arrangement to find out the most optimized solution to produce an efficient safe earthquake resistant building.

A commercial package ETABS has been utilized for analyzing high-rise building of 20 stories in different zones with respect to three types of soils. The result has been compared using tables & graph to find out the most optimized solution. Concluding remark will be made on the basis of this analysis & comparison tables.

INTRODUCTION

The purpose of earthquake prevention of buildings is to provide the structural safety and comfort by controlling the internal forces and displacement within the particular limits. The common method for protecting the structures against the destructive effects of earthquakes is to damp the seismic energy for limiting the seismic energy by the structural elements, thus providing the resistance against the earthquake. In spite of using this method for a certain level of protection, the structure could be damaged for real sometimes. Another method for protection of the structures against the earthquake is to isolate the building from the ground and/or to install seismic energy dissipating elements at the appropriate places of the building. With this method, better protection could be provided, by designing correctly against the earthquake and therefore significant structural damage level could be minimized.

The earthquakes have been carried on to be an important factor that threatens the social and economic future of the countries, as we can observe the results of them. Thus, it is insisted on the resolutions that minimize the seismic effects of the buildings should demonstrate a high performance level in the expected earthquakes. The seismic isolators and energy dissipating devices are seen to be effective solutions within this context, which are placed in the building appropriately to damp the seismic energy or placed between the foundation and vertical structural systems damping the seismic energy under the ground of the building, thus decreasing the effects of lateral loads on top floors. Application of earthquake protection systems in buildings whether will be constructed and were constructed -especially the historical ones-, increases the importance of these technologies.

Seismic isolation

Seismic isolation is a technique used to reduce the effects of earthquake ground shaking on structure, their components and protect them from damaging. In this technique we use some hardwires that I will describe later to reduce structures lateral movement (Drift).

Seismic isolation is one of the most important concepts for earthquake engineering which can be defined as separating or decoupling the structure from its foundation.

BASE ISOLATION SYSTEMS

The most extensively used methods today are the methods which are based on the separation of the building and the ground, allowing a horizontal movement on the foundations of the building/on the bearings of vertical structural members. These systems will be called base isolation systems in general. Since seismic isolators are placed between the superstructure and the ground or to separate certain parts of the building, this type of seismic isolation is also defined as external isolation.

Rubber Bearings

These systems also have steel laminated rubber types and steel laminated rubber types with lead nucleus, along with the ones made of rubber and neoprene. The natural and artificial rubber bearings, which were used in bridge bearings, have later been developed and have been named elastomeric bearings. These bearings, which are used as seismic isolators, are widely used.

a) Natural Rubber Bearing (NRB), b) elastomeric bearing device, c) Lead Rubber Bearing (LRB)

Friction pendulum bearings:

Friction pendulum systems are the most extensively used kinematic systems especially in base isolation. Pendulum system consists of a steel globe placed in two steel concave curved surface or a cylindrical member with global contact surfaces. In these parts special metals are used.

FIG.7.Cross section of a friction pendulum bearing Detail of a friction pendulum bearing

LITERATURE REVIEW

1. Anoop Mokha, M. C. Constantinou, Associate Member, ASCE, A. M. Reinhorn and Victor A. Zayas Members, ASCE A shake-table study of the friction-pendulum isolation system, installed in a six-story, quarter-scale, 52-kip model structure, is presented. Two bearing materials are studied, one with a peak friction coefficient of 0.075 and another of 0.095. In both cases, the isolation system has a rigid-body mode period of 1 sec. The isolated structure is found to be capable of withstanding strong earthquake forces of different frequency content.

2. Arathy S. and Manju P.M reported that Friction pendulum bearings (FPBs) are a type of base isolation technique which essentially detaches structures from the ground to help stabilize the building from the unstable ground motion. FPBs allow

IJTIMES-2018@All rights reserved 92

superstructures to rest atop two concave surfaces with a ball bearing as a buffer between the two surfaces. During an earthquake the bearings shift against the direction of earthquake, hence by keeping the building stable.

ISOLATION SYSTEMS

EVALUATION OF SEISMIC ISOLATION SYSTEMS

Seismic isolation is a technology that was developed in order to minimize the earthquake damage. It is a design method that is based on the principle of decreasing the earthquake energy affecting the structure by extending the structure period instead of increasing the resistance capacity of the building against the earthquake. In the buildings that are constructed by using this technology, the elastic behaviour of the building is ensured even during major earthquakes. Initially, the purpose was to prevent the collapsing of the buildings during an earthquake, but today, the designs that aim to maintain comfort in addition to earthquake security are on the foreground.

MODELLING

STOREY SHEAR

Story shear for loose soil in X-Direction

TABLES AND GRAPHS

Story shear for loose soil in Y-Direction

Story shear in medium soil

Story shear in Y-Direction

Story shear in Hard soil in X-Direction

Story shear in Hard soil in Y-Direction

Storey Shear values in Low Soil of Zone – V in X – Direction

Storey Shear values in Medium Soil of Zone – V in X – Direction

Storey Shear values in Medium Soil of Zone – V in Y – Direction

Storey Shear values in High Soil of Zone –V in X – Direction

Storey Shear values in High Soil of Zone – V in Y – Direction

STOREY MOMENT

Storey Moment values in Low Soil of Zone – II in X – Direction

Storey Moment values in Low Soil of Zone – II in Y – Direction

Moment values in Medium Soil of Zone – II in X – Direction

Storey Moment values in Medium Soil of Zone – II in Y – Direction

Storey Moment values in High Soil of Zone – II in X – Direction

Storey Moment values in High Soil of Zone – II in Y – Direction

Storey Moment values in Low Soil of Zone – V in X – Direction

Storey Moment values in Low Soil of Zone – V in Y – Direction

Storey Moment values in Medium Soil of Zone – V in X – Direction

Storey Moment values in Medium Soil of Zone – V in Y – Direction

Storey Moment values in High Soil of Zone –V in X – Direction

Storey Moment values in High Soil of Zone –V in Y – Direction

Torsion

Torsion values in Low Soil of Zone II

Torsion values in Medium Soil of Zone –II

Torsion values in High Soil of Zone –II

Torsion values in Low Soil of Zone –V

Torsion values in Medium Soil of Zone –V

Torsion values in High Soil of Zone –V

Story Drift Storey Drift values in Low Soil of Zone–II in X – Direction

Storey Drift values in Low Soil of Zone – II in Y – Direction

Storey Drift values in Medium Soil of Zone – II in X – Direction

Storey Drift values in Medium Soil of Zone – II in Y – Direction

Storey Drift values in High Soil of Zone – II in X – Direction

Storey Drift values in High Soil of Zone – II in Y – Direction

Storey Drift values in Low Soil of Zone – V in X – Direction

Storey Moment values in Low Soil of Zone – V in Y – Direction

Storey Drift values in Medium Soil of Zone – V in X – Direction

Storey Drift values in Medium Soil of Zone – V in Y – Direction

Storey Drift values in High Soil of Zone –V in X – Direction

GB RUB FIP

Storey Drift values in High Soil of Zone –V in Y – Direction

CONCLUSIONS

- 1. Storey shear decreased in both the directions when the building is damped with Friction Pendulum System followed by Lead Rubber Dampers in both the Zones (II&V) on all the soils (LS, MS, HS).
- 2. Storey Moment decreased in both the directions when the building is damped with Friction Pendulum System followed by Lead Rubber Dampers in both the Zones (II&V) on all the soils (LS, MS, HS).
- 3. Torsion decreased when the building is damped with Friction Pendulum System followed by Lead Rubber Dampers in both the Zones (II&V) on all the soils (LS, MS, HS).
- 4. Storey Drift decreased in both the directions when the building is damped with Friction Pendulum System followed by Lead Rubber Dampers in both the Zones (II&V) on all the soils (LS, MS, HS).
- 5. Optimum control of the parameters considered was observed when the building is damped with Friction Pendulum System followed by Lead Rubber Dampers. So from the work carried out it can be stated that Friction Pendulum System is the best supplemental damping system.

REFERENCES

- 1. ATC 17-1 (1993). "Proceedings of Seminar on Seismic Isolation, Passive Energy Dissipation, and Active Control," Applied Technology Council, Redwood City, California.
- 2. Aiken, I.D. and Kelly, J.M. (1990). "Earthquake Simulator Testing and Analytical Studies of Two Energy-Absorbing System for Multi-storey Structures," Report No. UCB/EERC-90/03, University of California at Berkeley.
- 3. Architectural Institute of Japan (1995). "Preliminary Reconnaissance Report of the 1995 Hyogoken Nanbu Earthquake. "
- 4. Bergman, D.M. and Hansen, R.D. (1993). "Viscoelastic Mechanical Damping Devices Tested at Real Earthquake Displacements," Earthquake Spectra, Vol. 9, No. 3, pp. 389417.
- 5. Chang, K.c., Soong, T.T., Oh, S.T. and Lai, ML. (1992). "Effect of Ambient Temperature on Viscoelastically Structures," Journal of Structural Engineering, ASCE, Vol. 1 18, No. 7, pp. 19551973.
- 6. Chang, K.C., Soong, T.T., Lai, M.L. and Nielson, E.J. (1993). "Viscoelastic Dampers as Energy Dissipation Devices for Seismic Applications," Earthquake, Spectra, Vol. 9, No. 3, pp. 371-387.
- 7. Chang, K.c., Soong, T.T., Oh, S.T. and Lai, ML. (1995). "Seismic Behavior of Steel Frame with Added Viscoelastic Dampers," Journal of Structural Engineering, ASCE, Vol. 121, No. 10, pp. 14181426.
- 8. Chang, K.C. Chen S.J. and Lai, M.L. (1996). "Inelastic Behavior of Steel Frames with Added Viscoelastic Dampers," Journal of Structural Engineering, ASCE, Vol. 122, No. 10, pp. 1178-1186.
- 9. Chang, K.c., Tsai, M.H., Chang, Y.H. and Lai, ML. (1998). "Temperature Rise effect of Viscoelastically Damped Structures under Strong Earthquake Ground Motions," The Chinese Journal of Mechanics, Vol. 14, No. 3, pp. 125- 136.