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UTILIZATION OF ADVANCE TECHNOLOGY AND TECHNIQUES IN ANALYSIS OF A TALL STRUCTURE

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Abstract:

Now-a-days, the architects often prohibit the widths of the columns so that more free space is available and for the good aesthetic look of the building without columns protruding out of the walls and corners. Advances in structural members and techniques to resist lateral forces are generally used now days to pretend more stable and safe structure.

In present work with the end goal to contrast fortified solid structure using traditional columns against structure designed with special shaped columns considering seismic loads on G+12 multi-story building having plan measurement 63.20mx29.50m m is displayed and dissected in Etab adaptation coordinated building outline programming. Proportionate static investigation and dynamic reaction range examination are performed on the structure.

Keywords: Structural stability, analysis, advance techniques, software's, columns, lateral forces.

Introduction:

Presently a-days, the designers regularly disallow the widths of the segments so more free space is accessible and for the great stylish look of the structure without sections projecting out of the dividers and corners.

Solid structures with non-rectangular uncommonly formed flimsy segments discovered as an option in contrast to the above said issue and it is discovered that non-rectangular uniquely molded dainty segments performs well basically with all investigation results inside satisfactory cutoff points. This sort of structure fulfills spatial necessities of corners just as crossing point of corners with the goal that no obvious edges or conspicuous segment would show up in the structures. This grows the real usable floor territory and more furniture can be put into the structures. Consequently, outline structure with uncommonly formed segment is appropriate blossoms, particularly for manor and multi-story structures. This paper clarifies the conduct of structures with Rectangular segments and structures with extraordinarily molded segments and estimations of different boundaries like story float, story relocation, story firmness and so on based on parallel burdens.

Basic plan is a science and specialty of understanding the conduct of auxiliary individuals exposed to burdens and planning them with economy alongside wellbeing, usefulness and as a strong structure. The current thesis work will manage such an investigation of basic individuals made of RCC as it is generally utilized due to its versatility. Section is fundamentally an auxiliary part allocated for conveying compressive burdens. It conveys hub loads from shafts and moves it to balance. The sections are recognized from multiple points of view and numerous sorts are watched. In view of the slimness proportion sections are called as short or long segments. The short segment falls flat by pulverizing and long segment bombs by clasping. Considering the stacking design there are pivotally stacked segment, hub segment with uniaxial bowing, hub segment with biaxial twisting. Segments carry on diversely under static and dynamic stacking conditions. The dynamic burden thought is must for places where the seismic movement is high. Thusly when seismic burdens are viewed as the joined methodology of pliability and quality must be applied. The breeze loads, snow loads, creep, shrinkage and temperature impacts are viewed as where they are essential. The snow load thought differs on nation to nation, and locale to area. It must be considered according to important plan codes of significant codes.

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The thought of wind stacks chiefly relies upon elevation of structure and the bearing and speed of the breeze in the locale. The segments ought to be intended to convey the breeze loads in such conditions. A portion of the complex mathematical conditions and building abnormalities requires test and systematic methodologies for various material properties and sectional properties. Such boundaries can't be legitimately secured by the structure code books and rules. The structural prerequisite leads the architect to consider various shapes and cross areas of the pillars, segments, plates and so on. This prompted distinctive cross segments of segments dependent on the spot and capacity, for example, L-shape, T shape, C-shape, +shape and so forth. Indeed, even the various states of segments are additionally by and by, for example, shaft, trapezoid, pinnacle and cross section. Every one of these segments of segments, for example, L-shape, T shape, C-shape, cross shape have made a change in basic designing by which they gives the simple answer for areas in extraordinary cases.

In the RCC structures, segments are auxiliary components which are dominatingly exposed to pivotal compressive powers, minutes, and moves all out burden from the super structure to sub-structure. Different states of the segments are utilized. Some normal shapes are square, rectangular, round sections and some extraordinary states of segments are L-molded, T-formed and in addition to (+) molded segments as appeared in figure 1.1 which are not regularly utilized yet gives more indoor space than usually utilized states of segments. Exceptional molded segments dodge conspicuous corners in a room which increments usable floor zone.

The fortified solid structure framework with uncommonly molded sections is a framework generally received in private structures because of no uncovered shafts and segments in the room. The practicality and comfort of strengthened cement (RC) outline with extraordinary molded sections in auxiliary structure and its positive assistance make it generally utilized in development industry.

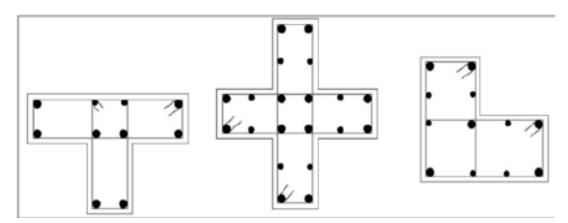


Fig 1 Specially shaped (T, +, Plus) columns with Longitudinal and lateral reinforcement.

Literature Review

Yuzhuo WANG et al (2019) The exploration paper introduced testing of three T-framed steel-fortified strong sections analyzed under high temperature and vertical burdens, to impersonate fire sway. The outcomes presumed that disappointment qualities, circulation of temperature field, vertical disfigurement credits and impenetrability to fire were likewise explored under different center tension extents and various flightiness. The test moreover showed that the breaks extended with the extension of center point pressure extent and unusualness. The harms of the web were severer than the rib. The breaks were for the unusual side on the whimsical side and by and large inclined parts in the model. The vertical augmentation ended up being progressively apparent as the eccentricism decreased. The impenetrability to fire reduced as the pivotal weight extent extended. Differentiated and, the impenetrability to fire of huge hub pressure models (the urgent weight extent was 0.6) was reduced by 57% than less hub pressure models (the center weight extent was 0.2). The impenetrability to fire reduced by about 30min as flightiness extended by 20mm.

Mary Paul V and Nisha Vargheese (2019) The examination paper dismembered Crisscrossed formed segments related by the binding bar, Single vertical steel plate with stiffeners, Double vertical steel plate, Effect of height, Effect of width and pivotal compressive lead. The attributes of the limited component examination were used on as far as possible conditions and material properties using ANSYS 16.2.

The results derived that Mono fragments related with twofold vertical steel plate had more burden passing on limit however Mono segments related by a binding bar had a less burden conveying limit. Mono sections related by single vertical steel plate with stiffeners have more weight passing on limit than restricting bars. Burden passing on limit oppositely propositional to the height of the fragments. Burden passing on limit depends on the width of the steel plate. The proportion of repression concrete expanded burden conveying limit.

Objectives of the study:

The main objectives of this study are to evaluate the performance of building with special shaped column. Followings are the specific objectives of this study.

- (1) To study the behavior of the building under lateral loads like seismic and wind loads.
- (2) Determination of deflections, storey drifts, storey stiffness and storey overturning moment of the buildings under wind and seismic conditions.
- (3) To compare the performance of the special shaped column building with building of rectangular columns.
- (4) To determine the effect of shaped column over stability of the structure.
- (5) To compare cost analysis of all the cases as per S.O.R.

Methodology:

For this research work following steps should be followed:

Step-1 Firstly literature survey should be done to determine the past research and Need of study.

Step-2 To prepare Sample of light weight concrete to determine its properties to be Assign in ETABS.

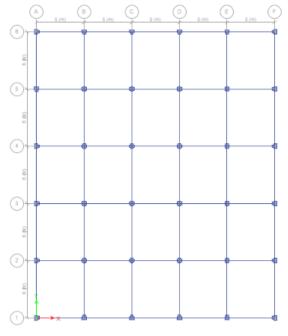


Fig 2 plan of structure

Step-3 To prepare modelling of a symmetrical building frame (G+12) using ETABS"17.

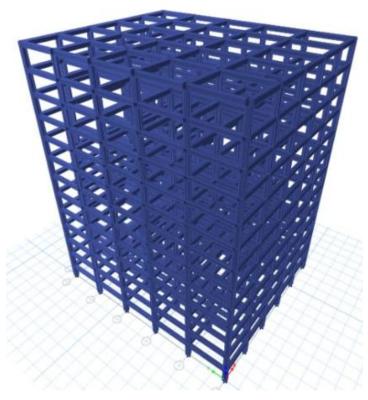


Fig 2 Model of the structure

Step-4 To assign properties and support conditions.

Joint Assignment - Restraints
Restraints in Global Directions
✓ Translation X ✓ Rotation about X
✓ Translation Y ✓ Rotation about Y
▼ Translation Z ▼ Rotation about Z
Fast Restraints

Fig 3 Assigning support conditions

Step 5 Defining and Assigning section Properties

General Data			
Property Name	TEE		
Base Material	M25 ~		
Notional Size Data	Modify/Show Notio	nal Size	
Display Color	Ch	ange	1
Notes	Modify/Show N	otes	
Design Type			
O No Check/Design	 General 	Steel Section	
 Concrete Column 	 Compositi 	e Column	
Concrete Column Check/De	sign		
Reinforcement to be	Checked		
	Designed		
O Reinforcement to be	Designed		
Reinforcement to be Define/Edit/Show Section	Designed		
	Section Designer		
	-	odifiers	
Define/Edit/Show Section	Section Designer Property M	odifiers et Modifiers	

Fig 4 (a) Defining Section Properties

perty Name Section Name	FSec1	
Base Material	M25	
Dase Material	MES	
perties		
Item		Value
Area, cm2		2100
AS2, cm2		1890.6
AS3, cm2		1890.6
133. cm4		357500
122. cm4		357500
S33Pos, cm3		14300
S33Neg. cm3		14300
S22Pos. cm3		14300
S22Neg, cm3		14300
R33, mm		130.5
R22, mm		130.5
Z33, cm3		23250
Z22, cm3		23250
J, cm4		606081.8
CG Offset 3 Dir, mr		0
CG Offset 2 Dir, mr		0
PNA Offset 3 Dir, n		0
PNA Offset 2 Dir, n	nm	0

Fig 4 (b) Defining Frame Section Properties

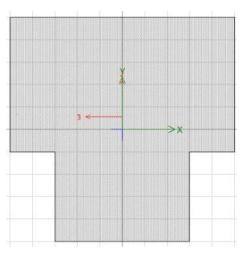


Fig 4 (c) Section Design T Shaped

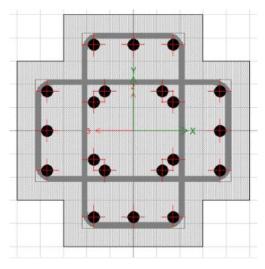


Fig 4 (d) Section design Plus Shaped

Step-6 To Assign lateral force (response spectrum) dynamic analysis as per I.S. 1893-Part-I: 2016.

		Self Weight	Auto	Clic	k To:
Load	Туре	Multiplier	Lateral Load		Add New Load
dynamic analysis	Seismic	▼ 0	IS1893 2002	-	Modify Load
Dead Live dynamic analysis	Dead Live Seismic	1 0	IS1893 2002		Modify Lateral Load
dynamic analysis	Seismic		151655 2002		Delete Load
Indian IS1893 :	2002 Seismic Loading				×
		ricity C	nce Factor, I	0.36 	•
Top Story	Story10		roximate Ct (m) =	
Bottom Sto	Base		gram Calculated		
		O Use	r Defined	Γ=	sec

Fig 5 Defining loading conditions

Step-7 To analyze the structure for dynamic loading.

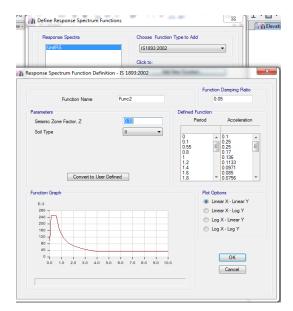


Fig 6 Dynamic analysis

Step-8 To compare the results of the structure.

Problem Formulation:

Type of structure	Ordinary moment resisting RC frame
Grade of concrete	M 40 (fck= 40 N/mm2)
Grade of reinforcing steel	Fe 415
Plan area	63.20mx29.50m
Number of stories	G+12
Total Height of Building	43.2 m
Floor height	3.6 m
Rectangle Shaped Column	230x 600mm
Plus Shape column	350mmx750mm
T Shape Square column	350mmx600mm
Beam size	500x300mm
Wall thickness	230mm
Thickness of Slab	200 m
Density of concrete	25N/ mm3
Live Load on Floor and roof	3 kN/mm2 and 1.5 kN/ mm2
Plan irregularity	T Shape and Plus Shape
Seismic Zone	II
Soil Condtion	Medium Soil
Floor Finish	1.0 kN/m2

Table 1 Details and Dimension of the Building Models

s. no	Description	Values
1	Material property	Values
2	Grade of concrete	M-25
3	Young's modulus of concrete, E _c	$2.17 \text{x} 10^4 \text{ N/mm}^2$
4	Poisson ratio	0.17
5	Tensile Strength, Ultimate steel	505 MPa
6	Tensile Strength, Yield steel	215 MPa
7	Modulus of Elasticity steel	193 - 200 GPa

Table 2 Material properties of structure

Analysis Result:

Max. Bending Moment:

Table 3: Max. Moment in KN-m

Moment in kN-m			
Storeys	Rectangular Column	T Shape	Plus Shape
storey12	826.76	734.17	566.81
storey11	814.58	705.63	553.84
storey10	789.44	677.09	540.87
storey9	770.21	648.55	527.9
storey8	719.3	620.01	514.93
storey7	698.67	591.47	501.96
storey6	675.2	562.93	488.99
storey5	629.1	534.39	476.02
storey4	559.89	505.85	463.05
storey3	578.9	477.31	450.08
storey2	524.99	468	437.11
storey1	510.22	461.05	424.14

Max. Shear Force

Table 4: Max. Shear Force in KN

	Shear force in kN-m				
Storeys	Rectangular Shape	T Shape	Plus Shape		
storey12	941.85	840.43	821.54		
storey11	923.25	813.87	797.91		
storey10	904.65	787.31	771.23		
storey9	886.05	760.75	748.9		
storey8	867.45	734.19	712.45		
storey7	848.85	707.63	694.766		
storeyб	830.25	681.07	672.85		
storey5	811.65	654.51	637.31		
storey4	793.05	627.95	619.21		
storey3	774.45	601.39	583.36		
storey2	755.85	574.83	554.45		
storey1	737.25	548.27	528.77		

Storey Displacement in mm

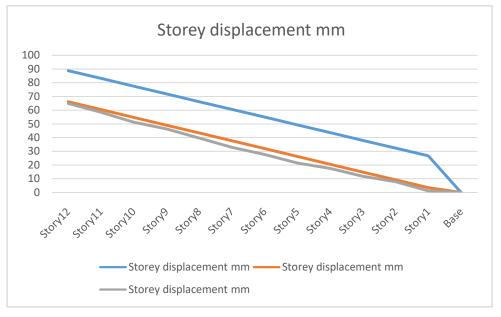


Fig 7: Storey Displacement

Conclusion:

Maximum Bending Moment:

In the chapter above, it is clearly observed that bending moment in structure using Rectangular column was 826.76 kN-m whereas structure using T shapes and Plus Shaped column showed less bending moment as 734.17 and 566.81 kN-m, thus Plus shaped case requires less reinforcement.

Maximum Shear force:

In above chapter it is observed that unbalance forces are maximum in rectangular case 941.85 kN whereas in Plus shaped case these are reduced to 840.43 kN which shows stability of the structure.

Maximum storey displacement:

It is observed that lateral stability is comparatively increased in structure with plus shaped column case comparing to structure using traditional rectangular column. In case of light weight structure displacement is minimised to 66.16 mm instead of 88.84 mm in bare frame.

Axial Force:

In the above chapter it is observed that there is very minute variation in axial force as it is considered for same loading condition in both the cases.

Future scope:

- i) In the proposed work high rise building is considered which can be increased to some more floors in future with variation in floor to floor height.
- ii) In this study seismic analysis is considered whereas in future study wind load can be considered.
- iii) In this study analysis is done using etabs whereas in future SAP2000 can be prefer for P-delta analysis.
- iv) In this study cost analysis is performed as per S.O.R. whereas in future one can adopt market rate for estimation.

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